

# Data report: strength characteristics of sediments from IODP Expedition 308, Sites U1322 and U1324<sup>1</sup>

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## Abstract

Triaxial strength experiments completed on specimens from Integrated Ocean Drilling Program Expedition 308 Sites U1322 and U1324 document internal friction angles from 17° to 29° and normalized undrained shear strength from 0.21 to 0.36 at maximum shear. Just prior to undrained shearing, lateral stress ratios for these triaxial specimens range from 0.50 to 0.73. In general, internal friction angle and normalized undrained shear strength decrease with increasing vertical consolidation stress. Lateral stress ratio does not vary systematically with vertical consolidation stress. All 18 specimens in this study were collected with the advanced piston corer (APC). Specimens from Site U1322 were from 27.3–72.9 meters below seafloor (mbsf), and those from Site U1324 were from 89.1–304 mbsf. For each specimen we completed a  $K_0$ -consolidated undrained triaxial test to provide data on the stress-strain behavior, undrained shear strength, internal friction angle, and secant modulus. Undrained shearing for each specimen was conducted at a vertical consolidation stress exceeding the hydrostatic vertical effective stress for that specimen.

## Introduction

Integrated Ocean Drilling Program (IODP) Expedition 308 was dedicated to understanding fluid flow, in situ pressure, and slope stability in rapidly deposited sediments of the northern Gulf of Mexico. The Ursa region was one of the focus areas for Expedition 308 because of high-quality multichannel seismic data illuminating depositional architecture (Sawyer et al., 2007), previous industry drilling that documented overpressure in shallow sand and mud (Ostermeier et al., 2001; Pelletier et al., 1999), and a series of stacked mass-transport complexes (MTCs) (see the “**Expedition 308 summary**” chapter). The Ursa region is ~210 km southeast of New Orleans, Louisiana (USA) in ~1000–1300 m water depth (Fig. F1). Sites U1322 and U1324 are separated by 12 km along a west-southwest–east-northeast transect (Fig. F1).

Three-dimensional seismic data over the Ursa region show a basin floor fan (Blue Unit) overlain by mud-dominated levee deposits (see the “**Expedition 308 summary**” chapter). These levee deposits are thick near Site U1324 and thin toward Site U1322. The stacked MTCs are confined within these levee deposits. Drilling was proposed in this region to understand how flow within the

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Blue Unit affects pressure and stability in the bounding mud. The overpressure profiles at Sites U1324 and U1322 have been characterized through direct measurements (Flemings et al., 2008) and laboratory experiments (Long et al.). We present constraints on the strength of sediments from these sites based on triaxial experiments.

Whole-round samples were used in detailed triaxial experiments to evaluate the stress-strain behavior, undrained strength, friction angle, and modulus. These results complement grain size analyses (Sawyer et al.), consolidation and hydraulic conductivity analyses (Long et al.) of sediment, and permeability analyses (Schneider et al.) of sediments from this region. Together these studies provide a full suite of geotechnical parameters which will help us understand coupling between flow, deformation, and stability and can be used for inputs into basin models.

## Laboratory testing methodology

All laboratory tests (Table T1) were conducted in accordance with the American Society for Testing and Materials (ASTM) standards. For tests where ASTM standards do not exist, the procedures followed the established Massachusetts Institute of Technology (MIT) geotechnical laboratory protocols.

### Radiography

Radiography allows the selection of the best quality material for advanced geotechnical testing. All whole-round samples were X-rayed following a procedure similar to ASTM standard D4452 (ASTM International, 2003). X-ray images were used to assess sample quality, presence of inclusions, general soil type, and variation in soil layering. Radiographs are available in Nelson et al.

### Specimen index properties

Water content is measured by taking the difference in the weight of a soil before and after oven drying and then dividing this difference by the oven-dried weight. For each experiment we provide the water content of the test specimen ( $w_n$ ) (Tables T2, T3). We also provide the initial void ratio ( $e_i$ ) of each specimen (Tables T2, T3). Void ratio is defined as the volume of voids divided by the volume of solids. From the void ratio and water content we calculate the initial saturation ( $S_i = w_n G_s / e_i$ ) for each specimen (Tables T2, T3). We assume a constant specific gravity of the solid grains ( $G_s = 2.78$ ) for these clay-rich samples. All variables are defined in Table T4.

## Undrained strength testing

Consolidation and strength properties were measured from the results of  $K_0$ -consolidated undrained ( $CK_0U$ ) triaxial tests on specimens from Sites U1322 and U1324. The MIT geotechnical laboratory has developed a standard method for performing  $CK_0U$  tests. In addition, ASTM standard D4767 (ASTM International, 2004) was used as a reference for the triaxial testing. Undrained strength testing can be divided into four stages. The first stage of the test involves sample preparation by trimming the specimen in a trimming jig using a wire saw. After the sample is trimmed (~1.75 cm radius; ~8 cm height), it is placed on the triaxial base with a nylon filter fabric and a porous stone placed on each end. Side drains were not used. Two thin, impermeable membranes are rolled over each specimen and sealed with three O-rings each at the top cap and bottom base of the triaxial chamber. The triaxial cell is then filled with silicon oil and tightly sealed. Distilled water was used as the fluid in the drainage system, which is connected to the top and base of the specimen.

Backpressure saturation is the second stage of the test. This phase ensures full saturation of the specimen. To do this, a modest pressure is applied to dissolve any air bubbles in the specimen. Next, a small, isotropic effective stress is applied to the specimen such that there is minimal to no axial strain. This effective stress is applied to seat the specimen in the triaxial cell. For the specimens from the Ursa region, the applied effective stress ranged from 16 to 76 kPa (Table T2). This isotropic effective stress is maintained while the axial stress and cell pressure are increased incrementally by the same value. To test for specimen saturation, the drainage lines are closed and the axial stress and cell pressure are increased incrementally and the  $B$  value (ASTM International, 2004) is measured. A  $B$  value of 0.98–1.00 is desired; however, it was not achieved in all experiments (Table T2). After the  $B$  value is measured, the drainage lines are opened for the consolidation phase of the test.

The third test stage is  $K_0$  consolidation. During  $K_0$  consolidation the specimen is consolidated one-dimensionally in the axial direction (i.e., no radial strain) following the SHANSHEP testing technique (Ladd, 1986).  $K_0$  consolidation allows vertical strain on the specimen but maintains the radius of the specimen. This simulates burial of the sediment in a confined basin where sediments deform vertically but are confined laterally and do not strain in the lateral direction.  $K_0$  is the ratio of the radial effective stress to the vertical effective stress required to main-

tain no radial strain ( $K_o = \sigma_r'/\sigma_a'$ ). We define  $K_o$  at the maximum vertical consolidation stress ( $\sigma_{vc}'$ ) as the consolidation lateral stress ratio ( $K_c$ ) (Table T2).  $K_o$  consolidation rates are provided in Table T2. After reaching the desired consolidation stress, total vertical stress, cell pressure, and pore pressure were held constant for a set time ( $t_s$ ) to allow excess pore pressure to dissipate and to allow some secondary compression (Table T2). For all specimens the maximum vertical consolidation stress exceeds the in situ effective vertical stress to ensure the specimen is on the primary consolidation path.

The final stage of the test is undrained shearing. Prior to starting the shear, a leak check is performed by closing the drainage valves for 30 min. During this time, the backpressure should remain constant. After the leak check, the specimen is sheared with the drainage lines closed. Shearing with the drainage lines closed prevents any fluid drainage during shear, maintains a constant volume of the specimen, and allows us to define the undrained strength parameters for each specimen. The shear rates are provided in Table T3. Positive shear strain data indicate a compression test, whereas negative shear strain data indicate an extension test.

## Experimental results

All specimen locations, depths, and corresponding triaxial experiments are provided in Table T1. Table T2 summarizes the consolidation phase of each experiment, and Table T3 provides information on the shear phase of each experiment. Complete experimental data are provided in “[Supplementary material](#).”

### Consolidation results

We describe the initial conditions and consolidation parameters for each specimen (Table T2). The specimen data define the initial conditions of the specimen including  $w_n$ , total density ( $\rho_t$ ),  $e_i$ , and  $S_r$ . Specimen water contents ranged from 28% to 50%, and initial void ratios ranged from 0.84 to 1.4.  $G_s$  was assumed to be 2.78. Initial consolidation conditions define the specimen and cell conditions prior to the start of  $K_o$  consolidation.  $\sigma_i'$  is the initial vertical effective stress applied to the specimen and  $\varepsilon_a$  is the axial strain after this initial stress is applied.  $u_b$  is the backpressure applied to the specimen.  $\varepsilon_{vol}$  is the total volume strain to the specimen during backpressure saturation.  $B$  is the pore pressure parameter ( $B = \Delta u / \Delta \sigma_r$ ).

The consolidation results portion of the table describes the parameters of the  $K_o$  consolidation. The

strain rate ( $\varepsilon_a/h$ ) defines the constant rate of axial strain maintained during consolidation. Strain rates for each experiment were constant but varied from 0.05%/h to 0.23%/h between experiments (Table T2). The specimen conditions at maximum stress define the conditions at the end of primary  $K_o$  consolidation. These include  $\varepsilon_a$ ,  $\varepsilon_{vol}$ , maximum effective vertical stress ( $\sigma_{vm}'$ ), and lateral stress ratio ( $K_c$ ) at  $\sigma_{vm}'$ . The time allowed for secondary compression after reaching  $\sigma_{vm}'$  is  $t_s$ . The final parameters define the conditions at the end of secondary compression including  $\sigma_{vc}'$  and overconsolidation ratio (OCR). All experiments were controlled to have an OCR = 1 such that  $\sigma_{vc}'$  was the greatest effective stress to which the specimen was exposed. Prior to initial shearing, the consolidation stress ratio ( $K_c$ ) for the specimens ranged from 0.50 to 0.73 (Fig. F2).

Standard results from the  $K_o$ -consolidation phase include the stress-strain behavior, total work done to the specimen, and the lateral stress ratio (Fig. F3). From the stress-strain data, basin model inputs can be constrained and compression behavior can be defined as is done for constant-rate-of-strain consolidation experiments (e.g., Long et al.). The total work data can be used to infer preconsolidation stress for each specimen (Long et al.; Becker et al., 1987). Work is calculated as

$$\text{work} = \frac{\sigma_i - \sigma_{i-1}}{2} \ln\left(\frac{1 - \varepsilon_{i-1}}{1 - \varepsilon_i}\right).$$

Example  $K_o$ -consolidation phase data are provided in Table T5. Complete  $K_o$ -consolidation phase data are provided as plots and tables in the CONSOL folder in “[Supplementary material](#).”

### Strength results

The shear phase conditions and results are provided in Table T3. The specimen data are identical to those in the consolidation phase (Table T2). The shear phase conditions provided in Table T3 define the strain rate for shearing ( $\varepsilon_a/h$ ) and the stress conditions ( $K_c$ ,  $\sigma_{vc}'$ , and OCR) prior to shearing. The next section of the table provides the strength results at maximum shear and obliquity (Table T3). Obliquity refers to the ratio of the normalized shear stress ( $q$ ) to the normalized mean effective stress ( $p'$ ). The normalized undrained strength ranges from 0.21 to 0.36, whereas the friction angle ranges from 17° to 29° at maximum shear (Fig. F2; Table T3). Both properties show a relation to the vertical consolidation stress prior to shearing.

Example data from the shear phase are shown in Figure F4 and Table T6. The summary plots include normalized shear stress ( $q/\sigma_{vc}'$ ) versus  $\varepsilon_a$ , internal friction

angle ( $\phi$ ) versus  $\varepsilon_a$ , normalized excess pore pressure ( $\Delta u_e/\sigma_{vc}'$ ) and normalized shear-induced pore pressure ( $\Delta u_s/\sigma_{vc}'$ ) versus  $\varepsilon_a$ , and normalized stress path ( $q/\sigma_{vc}'$  versus  $p'/\sigma_{vc}'$ ) for each test. The following equations define  $p'$ ,  $q$ ,  $\phi$ , secant modulus ( $E$ ), and pore pressure parameter ( $A$ ), which are key parameters in the shear-phase results. All variables are defined in Table T4.

$$p' = \frac{\sigma_a' + \sigma_r'}{2}$$

$$q = \frac{\sigma_a - \sigma_r}{2}$$

$$\phi = \sin^{-1}\left(\frac{q}{p'}\right)$$

$$E = \frac{2\Delta q}{\varepsilon}$$

$$A = \frac{u_e - \Delta\sigma_r}{\Delta\sigma_a - \Delta\sigma_r}$$

Complete data from the undrained shearing portions of the triaxial tests are in the SHEAR folder in “[Supplementary material](#).”

## Summary

Triaxial strength experiments on 18 specimens from Sites U1322 and U1324 show that internal friction angle and normalized undrained shear strength generally decrease with increasing vertical consolidation stress. Internal friction angle ranges from 17° to 29° and normalized undrained shear strength ranges from 0.36 to 0.21 at maximum shear for vertical consolidation stresses from 230 to 1840 kPa. Lateral stress ratios do not exhibit any relation with vertical consolidation stress and range from 0.50 to 0.73 prior to undrained shearing. In conjunction with other physical properties analyses, these strength data help define a complete geotechnical characterization of sediments from the Ursa region.

## Acknowledgments

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were provided by the Integrated Ocean Drilling Program (IODP). Funding for this research was provided by the Consortium for Ocean Leadership.

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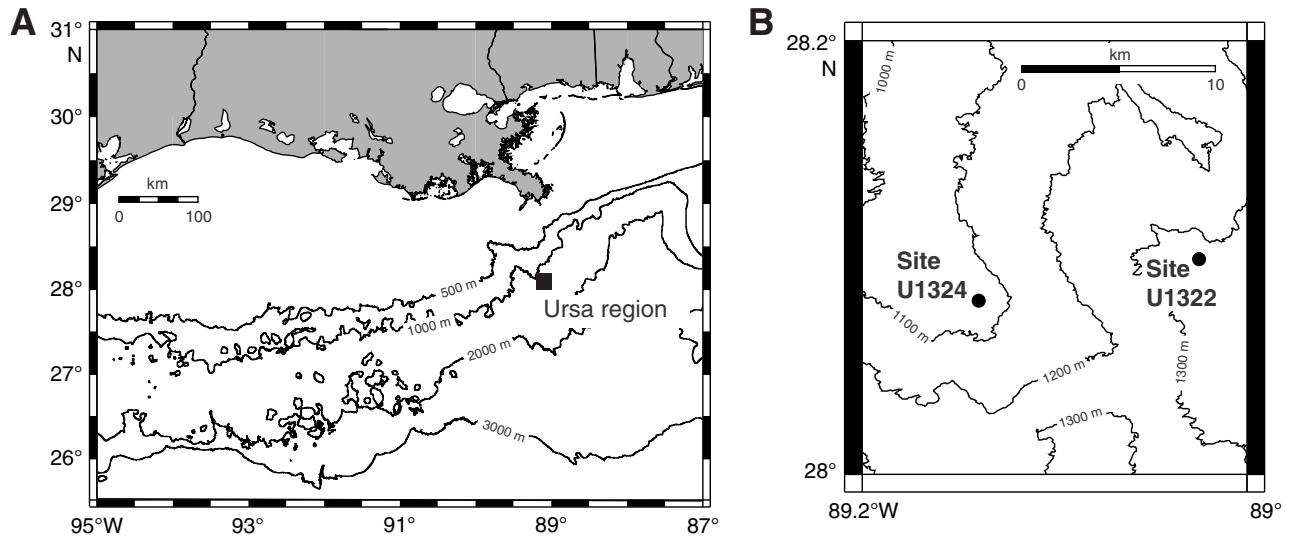
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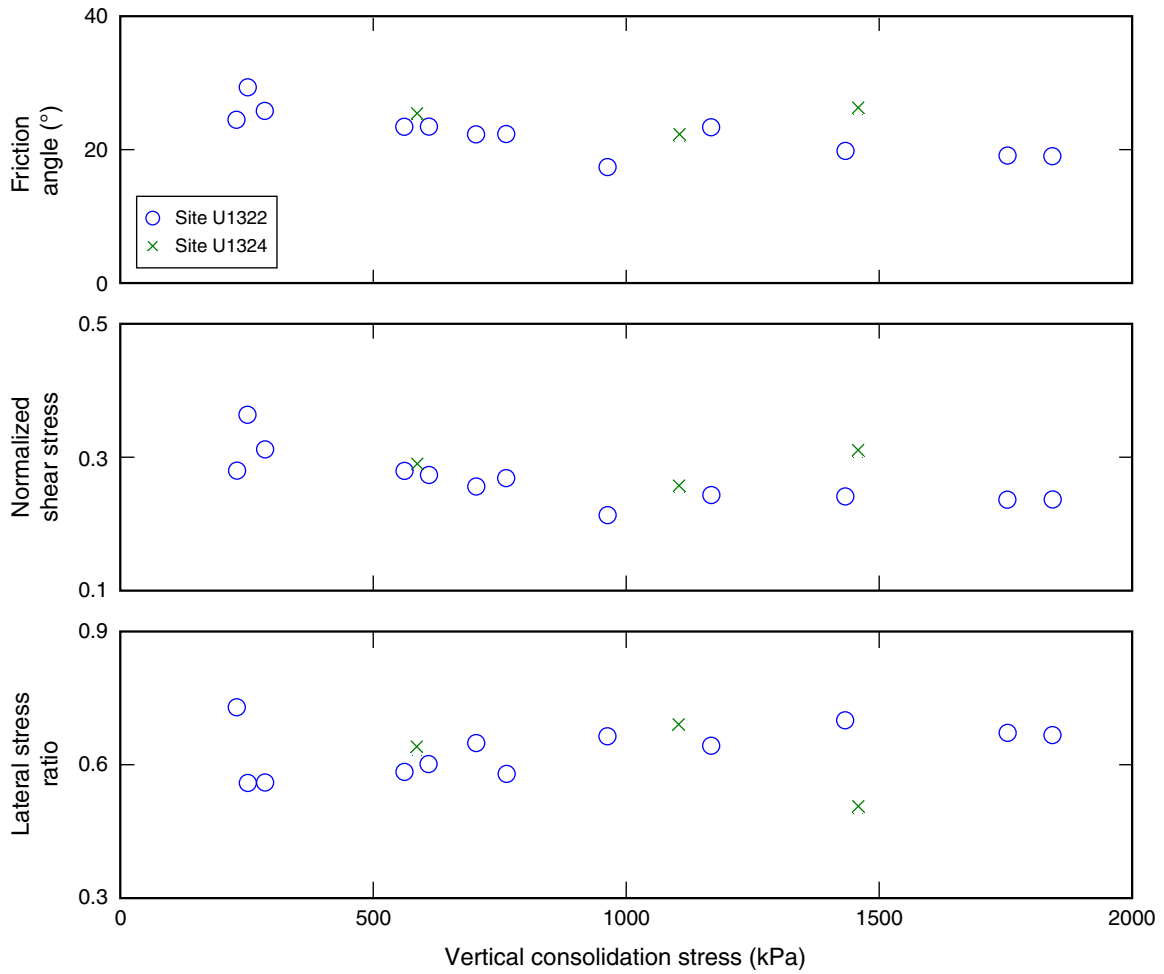
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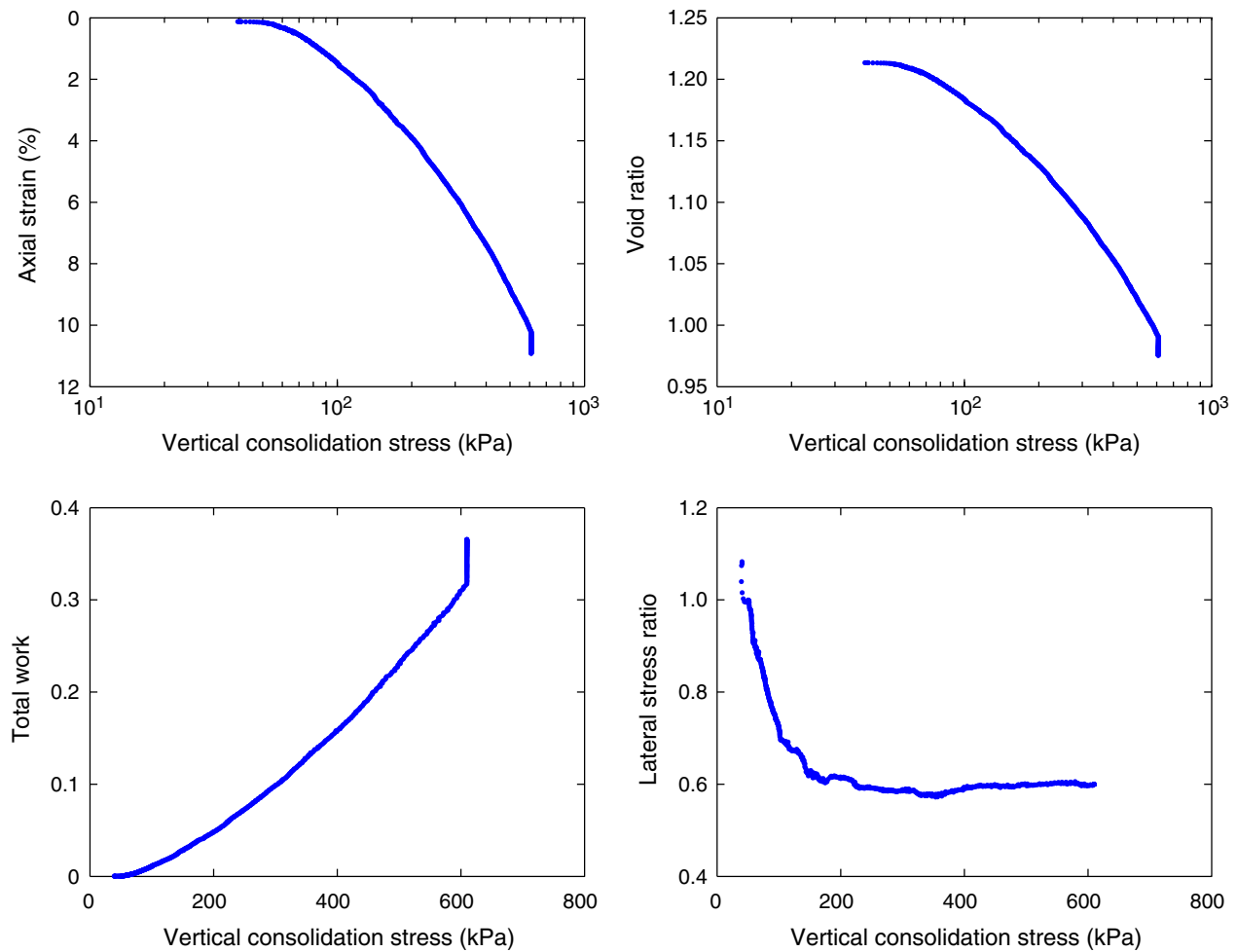
**Figure F1.** A. Regional basemap showing the location of the Ursa region, northern Gulf of Mexico. B. Sites U1322 and U1324 in Ursa region of the Mississippi Canyon.



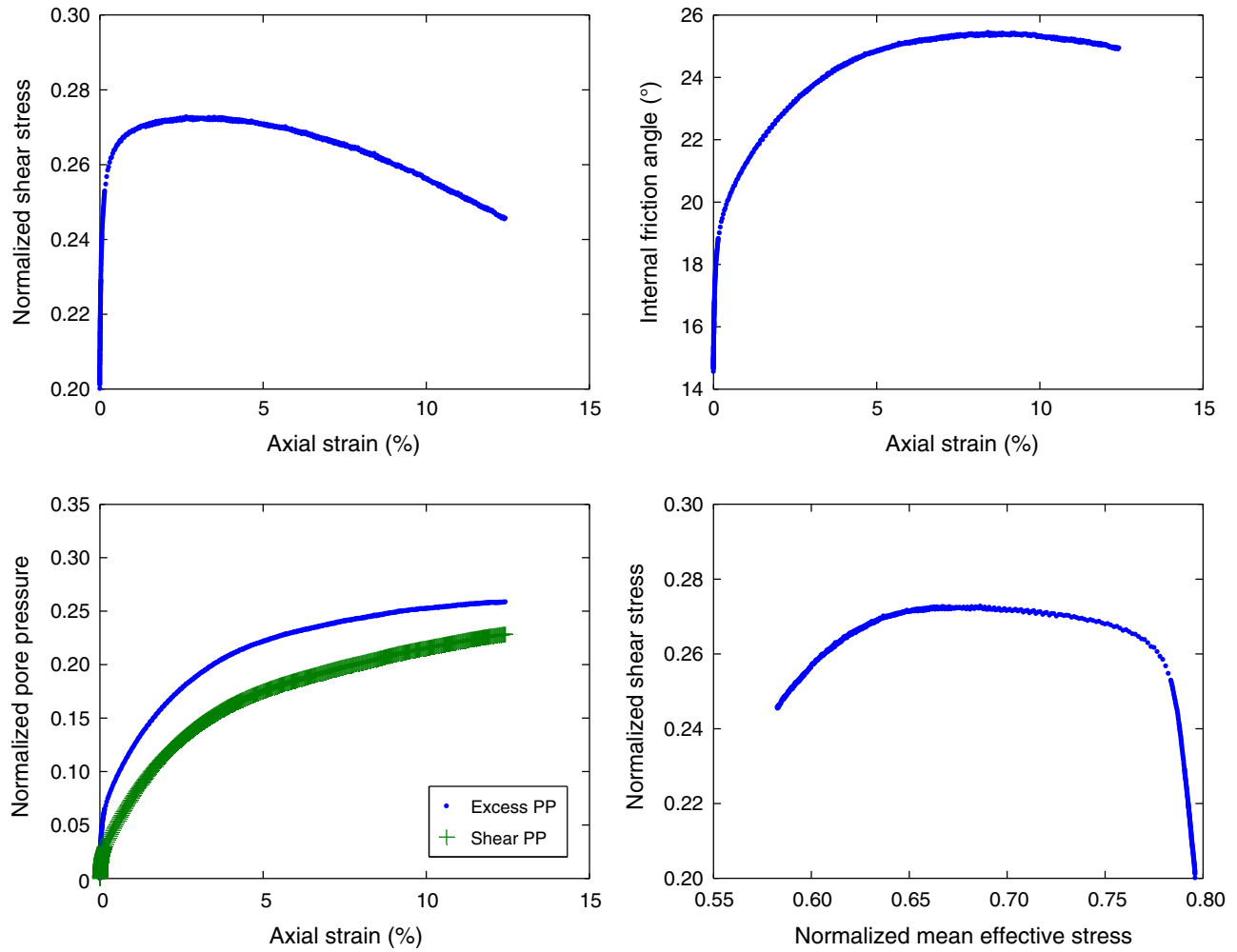
**Figure F2.** Summary of internal friction angle, normalized shear stress, and lateral stress ratio as a function of maximum vertical consolidation stress. Data are from Tables T2 and T3.



**Figure F3.** Example data from the consolidation phase of triaxial experiment TX729. Axial strain and void ratio vs. vertical consolidation are plotted on a logarithmic scale that can be used to define stress-strain behavior. Total work and lateral stress ratio ( $K_0$ ) vs. vertical consolidation stress are plotted on a linear scale. Work data can be used for interpreting in situ stress conditions.  $K_0$  data define how horizontal stress changes during uni-axial (no radial strain) loading.



**Figure F4.** Example data from the shear phase of triaxial experiment TX729. Axial strain is during undrained shearing. PP = pore pressure.





**Table T1.** Summary of samples used in triaxial experiments.

Hole, core, section	Depth (mbsf)	Test	Comments
308-			
U1322D-1H-2	42.9	TX725	
U1322D-2H-2	72.9	TX728	
U1322D-2H-2	72.7	TX729	
U1324B-10H-7	89.2	TX730	
U1324B-10H-7	88.9	TX734	
U1322B-4H-3	27.3	TX735	No void ratio data; included second shear
U1322D-2H-2	72.6	TX736	No void ratio data; included second shear
U1324B-10H-7	89.1	TX737	No void ratio data
U1324C-6H-3	304.0	TX763	
U1324B-18H-6	161.6	TX764	
U1322D-1H-2	42.6	TX767	
U1324C-6H-3	303.9	TX770	
U1322D-1H-2	42.7	TX773	
U1322D-1H-3	44.2	TX774	
U1322D-1H-3	44.1	TX775	
U1322D-1H-3	44.0	TX776	No consolidation data
U1322D-1H-3	43.9	TX778	No void ratio data
U1322D-1H-3	43.8	TX779	No void ratio data



**Table T2.** Summary of consolidation phase of triaxial experiments. (See table note.)

Hole, core, section	Test number	Depth (mbsf)	Specimen data					Test conditions					Consolidation results											
			$w_n$ (%)	$\rho_t$ (g/cm <sup>3</sup> )	$e_i$	$S_l$ (%)	$G_s$	$\sigma'_i$ (kPa)	$\varepsilon_a$ (%)	$u_b$ (kPa)	$B$	$\varepsilon_{vol}$ (%)	General			At maximum stress				At preshear				
													$\varepsilon_a/h$ (%/h)	$\varepsilon_a$ (%)	$\varepsilon_{vol}$ (%)	$\sigma'_{vm}$ (kPa)	$K_c$	$t_s$ (h)	$\varepsilon_a$ (%)	$\varepsilon_{vol}$ (%)	$K_c$	$\sigma'_{vc}$ (kPa)	OCR	$t_s$ (h)
308-																								
U1322B-4H-3	TX735	27.3	50.77	1.734	1.417	99.6	2.78	16.0	0.11	198	0.97	-0.64	0.23	10.33	10.78	229.8	0.73	34.0	10.33	10.78	0.730	229.8	1	34.0
Second shear	TX735							67.7	9.67	284		10.82	0.23	17.63	23.88	1164	0.989	45.0	17.63	23.88	0.989	1164	1	45.0
U1322D-1H-2	TX767	42.7	43.59	1.765	1.262	96.0	2.78	27.5	-0.21	296	0.86	-2.06	0.11		11.73	511	0.6	22.4		11.73	0.600	511	1	22.4
U1322D-1H-2	TX773	42.8	44.79	1.765	1.280	97.3	2.78	18.7	-0.61	209			0.16	20.38	20.94	1433	0.7	30.9	20.80	20.94	0.7	1433	1	30.9
U1322D-1H-2	TX725	42.9	44.00	1.780	1.249	98.0	2.78	46.0	0.06	294	0.90	-0.90	0.13	14.59	14.31	963.4	0.663	38.7	14.59	14.31	0.663	963.4	1	38.7
U1322D-1H-3	TX779	43.8						32.4	-0.39	206			0.24	21.45	21.59	1843	0.666	31.5	21.45	21.59	0.666	1843	1	31.5
U1322D-1H-3	TX778	43.9						33.2	-0.11	285			0.21	6.45	6.38	285.9	0.56	19.5	6.45	6.38	0.560	285.9	1	19.5
U1322D-1H-3	TX776	44.0								214			0.2	21.00	21.10	1754	0.672		21.00	21.10	0.672	1754	1	
U1322D-1H-3	TX775	44.1	40.27	1.769	1.204	92.9	2.78	40.4	-0.09	199			0.23	11.05	11.04	561.4	0.583	26.3	11.05	11.04	0.583	561.4	1	26.3
U1322D-1H-3	TX774	44.2	40.33	1.780	1.192	94.1	2.78	24.2	-0.82	189			0.2	15.04	15.05	763.3	0.579	20.7	15.04	15.05	0.579	763.3	1	20.7
U1322D-2H-2	TX736	72.6	40.77	1.826	1.143	99.2	2.78	17.0	-0.26	197	0.94	-1.83	0.2	6.08	6.19	252.1	0.558	34.7	6.08	6.19	0.558	252.1	1	34.7
Second shear	TX736							76.1	5.39	261	6.19		0.16	14.64	15.45	1168	0.642	3.7	14.64	15.45	0.642	1168	1	3.7
U1322D-2H-2	TX728	72.9	39.54	1.798	1.157	95	2.78	28.0	0.19	193	0.93	-2.08	0.05	11.38	11.33	703.8	0.648	54.0	11.38	11.33	0.648	703.8	1	54.0
U1322D-2H-2	TX729	72.7	42.76	1.791	1.216	97.7	2.78	44.0	-0.16	197	0.84	-0.84	0.07	10.92	10.87	610	0.6	45.3	10.92	10.87	0.600	610.0	1	45.3
U1324B-10H-7	TX737	89.1	38.90	1.796	1.149	94.1	2.78	50.0	0.06	178	0.84	-0.73	0.21	14.08	14.36	1105	0.691	56.5	14.08	14.36	0.691	1105	1	56.5
U1324B-10H-7	TX730	89.2	37.35	1.859	1.054	98.5	2.78	29.0	0.08	187	0.84		0.07	9.94	9.88	586.8	0.638	43.0	9.94	9.88	0.638	586.8	1	43.0
U1324B-18H-6	TX764	161.6	29.48	1.952	0.844	97.1	2.78	37.6	-0.03	294	0.86	-2.33	0.19	11.34	11.19	1461	0.494	66.9	11.34	11.19	0.494	1461	1	66.9
U1324C-6H-3	TX770	303.9	28.34	1.944	0.835	94.3	2.78	32	-0.54	295	0.88	-5.56	0.17	8.67	8.56	1458	0.507	34.9	8.67	8.56	0.507	1458	1	34.9
U1324C-6H-3	TX763	304.0	28.94	1.918	0.869	92.6	2.78	37.7	-0.25	293	0.92	-5.52	0.12	8.20	8.10	1418	0.509	31.9	8.20	8.10	0.509	1418	1	31.9

Note: See Table T4 for heading definitions.



**Table T3.** Summary of shear phase of triaxial experiments. (See table notes.)

Hole, core, section	Test number	Depth (mbsf)	Specimen Data					Test conditions					Triaxial results														
			$w_n$ (%)	$\rho_t$ (g/cm <sup>3</sup> )	$e_i$	$S_1$ (%)	$G_s$	$\epsilon_a/h$ (%/h)	$K_c$	$\sigma_{vc}'$ (kPa)	OCR	At maximum shear						At maximum obliquity									
												$\epsilon_a$ (%)	$q/\sigma_{vc}'$	$\Delta u_e'/\sigma_{vc}'$	$\Delta u_d'/\sigma_{vc}'$	$p'/\sigma_{vc}'$	$q/p'$	$\phi$ (°)	A	$\epsilon_a$ (%)	$q/\sigma_{vc}'$	$\Delta u_e'/\sigma_{vc}'$	$\Delta u_d'/\sigma_{vc}'$	$p'/\sigma_{vc}'$	$q/p'$	$\phi$ (°)	A
308-																											
U1322B-4H-3	TX735	27.3	50.77	1.734	1.417	99.6	2.78	0.66	0.730	229.8	1	5.00	0.279	0.336	0.238	0.673	0.415	24.5	1.160	4.98	0.279	0.336	0.238	0.673	0.415	24.5	1.16
Second shear	TX735							-0.58	0.989	1164	1	-11.5	-0.286	0.051	0.245	0.652	-0.438	-26.0	-0.090	-11.5	-0.286	0.051	0.245	0.652	-0.438	-26	-0.09
U1322D-1H-2	TX767	42.6	43.59	1.765	1.262	96.0	2.78	-0.42	0.600	511	1	-18.3	-0.240	-0.110	0.183	0.476	-0.505	-30.4	0.125	-16.94	-0.239	-0.105	0.187	0.472	-0.507	-30.5	0.12
U1322D-1H-2	TX773	42.7	44.79	1.765	1.28	97.3	2.78	0.48	0.700	1433	1	3.15	0.241	0.231	0.170	0.713	0.339	19.8	1.260	8.57	0.232	0.296	0.241	0.638	0.363	21.3	1.81
U1322D-1H-2	TX725	42.9	44.00	1.780	1.249	98.0	2.78	0.53	0.663	963.4	1	2.35	0.213	0.159	0.117	0.712	0.298	17.4	1.260	-8.97	-0.163	0.003	0.239	0.465	-0.352	20.6	-0.05
U1322D-1H-3	TX779	43.8						0.61	0.666	1843	1	3.11	0.237	0.170	0.124	0.726	0.326	19.0	1.230	9.73	0.225	0.240	0.202	0.644	0.349	20.41	2.1
U1322D-1H-3	TX778	43.9						0.48	0.560	285.9	1	5.18	0.310	0.155	0.095	0.714	0.435	25.8	0.860	7.7	0.376	0.166	0.108	0.700	0.537	32.5	0.95
U1322D-1H-3	TX776	44.0						0.51	0.672	1754	1	3.35	0.236	0.187	0.140	0.721	0.328	19.1	1.310	9.41	0.226	0.252	0.212	0.645	0.350	20.5	2.07
U1322D-1H-3	TX775	44.1	40.27	1.769	1.204	92.9	2.78	0.36	0.583	561.4	1	3.34	0.280	0.157	0.109	0.704	0.397	23.4	1.090	5.08	0.276	0.177	0.131	0.681	0.405	23.9	1.29
U1322D-1H-3	TX774	44.2	40.33	1.780	1.192	94.1	2.78	0.50	0.579	763.3	1	2.70	0.269	0.138	0.100	0.708	0.380	22.3	1.190	6.39	0.262	0.185	0.151	0.654	0.400	23.6	1.81
U1322D-2H-2	TX736	72.6	40.77	1.826	1.143	99.2	2.78	0.55	0.558	252.1	1	4.87	0.363	0.179	0.088	0.740	0.491	29.4	0.660	-4.98	-0.259	-0.154	0.171	0.45	-0.577	-35.2	0.158
Second shear	TX736							0.48	0.642	1168	1	3.94	0.242	0.264	0.220	0.613	0.395	23.3	2.040	7.41	0.237	0.295	0.255	0.577	0.411	24.3	2.49
U1322D-2H-2	TX729	72.7	42.76	1.791	1.216	97.7	2.78	0.47	0.600	610.0	1	2.65	0.273	0.182	0.134	0.686	0.397	23.4	1.260	8.41	0.263	0.246	0.205	0.612	0.430	25.4	1.96
U1322D-2H-2	TX728	72.9	39.54	1.798	1.157	95.0	2.78	0.53	0.648	703.8	1	3.08	0.255	0.216	0.169	0.676	0.377	22.2	1.510	8.31	0.241	0.287	0.249	0.591	0.408	24.1	2.5
U1324B-10H-7	TX737	89.1	38.90	1.796	1.149	94.1	2.78	0.52	0.691	1105	1	4.79	0.258	0.262	0.201	0.679	0.380	22.3	1.300	10.34	0.249	0.32	0.257	0.62	0.402	23.7	1.69
U1324B-10H-7	TX730	89.2	37.35	1.859	1.054	98.5	2.78	0.51	0.638	586.8	43	7.23	0.291	0.253	0.180	0.673	0.432	25.6	1.160	9.86	0.289	0.261	0.189	0.664	0.435	25.8	1.21
U1324B-18H-6	TX764	161.6	29.48	1.952	0.844	97.1	2.78	-0.53	0.494	1461	1	-12.4	-0.181	-0.011	0.278	0.323	-0.561	-34.1	0.013	-12	-0.181	-0.008	0.280	0.321	-0.562	-34.2	0.01
U1324C-6H-3	TX770	303.9	28.34	1.944	0.835	94.3	2.78	0.35	0.507	1458	1	2.80	0.310	0.120	0.078	0.701	0.443	26.3	0.954	5.37	0.308	0.132	0.091	0.687	0.448	26.6	1.08
U1324C-6H-3	TX763	304.0	28.94	1.918	0.869	92.6	2.78	-0.28	0.509	1418	1	-6.35	-0.168	-0.019	0.254	0.364	-0.461	-27.4	0.020	-6.7	-0.166	-0.009	0.264	0.354	-0.469	-28.0	0.01

Notes: Negative values in TX735, TX767, TX736, TX764, and TX763 represent extensional triaxial tests; all other tests were compressional tests. See Table T4 for heading definitions.

Table T4. Nomenclature.

Variable	Definition	Dimension	SI unit
$A$	Pore pressure parameter	Dimensionless	—
$B$	B value	Dimensionless	—
$E$	Secant modulus	M/LT <sup>2</sup>	kPa
$G_s$	Specific gravity of solid grains	Dimensionless	—
$e_i$	Initial void ratio	Dimensionless	—
$K_c$	Consolidation lateral stress ratio	Dimensionless	—
$K_o$	In situ lateral stress ratio	Dimensionless	—
OCR	Overconsolidation ratio	Dimensionless	—
$p'$	Mean effective stress	M/LT <sup>2</sup>	kPa
$q$	Shear stress	M/LT <sup>2</sup>	kPa
$S_i$	Initial saturation	Dimensionless	%
$t_s$	Time for secondary compression	T	h
$u_b$	Backpressure	M/LT <sup>2</sup>	kPa
$w_n$	Specimen water content	Dimensionless	%
$u_e$	Excess pore pressure	M/LT <sup>2</sup>	kPa
$u_s$	Shear induced pore pressure	M/LT <sup>2</sup>	kPa
$\epsilon_a/h$	Strain rate	1/T	%/h
$\epsilon_a$	Axial strain	Dimensionless	—
$\epsilon_{vol}$	Volumetric strain	Dimensionless	—
$\rho_t$	Total density	M/L <sup>3</sup>	g/cm <sup>3</sup>
$\phi$	Friction angle	Degrees	degrees
$\sigma_a'$	Vertical effective stress	M/LT <sup>2</sup>	kPa
$\sigma_i'$	Initial effective stress	M/LT <sup>2</sup>	kPa
$\sigma_r'$	Radial effective stress	M/LT <sup>2</sup>	kPa
$\Delta\sigma_r$	Radial stress change	M/LT <sup>2</sup>	kPa
$\Delta u$	Pore pressure change	M/LT <sup>2</sup>	kPa
$\sigma_{vm}'$	Maximum vertical consolidation stress	M/LT <sup>2</sup>	kPa
$\sigma_{vc}'$	Vertical consolidation stress	M/LT <sup>2</sup>	kPa

Table T5. Example data from consolidation phase of triaxial experiment TX729. (See table notes.)

Time (s)	$\epsilon_a$ (%)	$\sigma_a'$ (kPa)	$\sigma_r'$ (kPa)	$p'$ (kPa)	$q$ (kPa)	$\epsilon_{vol}$ (%)	$e$	$K_o$	Work	Area (cm <sup>2</sup> )	$u_b$ (kPa)
0	0.129	40.608	43.967	42.287	-1.679	0.129	1.213	1.083	0.000	9.931	197.322
41	0.129	40.919	44.131	42.525	-1.606	0.129	1.213	1.078	0.000	9.931	197.271
221	0.129	40.744	44.004	42.374	-1.630	0.129	1.213	1.080	0.000	9.931	197.233
401	0.128	40.043	43.008	41.526	-1.483	0.130	1.213	1.074	0.000	9.931	197.132
581	0.128	39.704	41.267	40.486	-0.782	0.129	1.213	1.039	0.000	9.931	197.195
761	0.130	40.838	41.467	41.153	-0.314	0.129	1.213	1.015	0.000	9.931	197.601
941	0.133	42.755	42.843	42.799	-0.044	0.130	1.213	1.002	0.000	9.931	197.297
1121	0.136	44.468	44.279	44.374	0.094	0.133	1.213	0.996	0.000	9.931	197.351
1301	0.140	46.020	45.816	45.918	0.102	0.137	1.213	0.996	0.000	9.931	197.347
1481	0.143	47.265	47.012	47.138	-0.126	0.140	1.213	0.995	0.000	9.931	197.360
1661	0.148	48.396	48.144	48.270	0.126	0.145	1.213	0.995	0.000	9.931	197.385
1841	0.153	49.603	49.378	49.490	0.113	0.150	1.213	0.995	0.000	9.931	197.347
2021	0.157	50.396	50.320	50.358	0.038	0.155	1.213	0.998	0.000	9.931	197.347
2201	0.161	50.767	50.734	50.751	0.017	0.159	1.213	0.999	0.000	9.931	197.389
2381	0.166	51.182	51.085	51.133	0.049	0.163	1.213	0.998	0.000	9.931	197.322
2561	0.171	51.846	51.750	51.798	0.048	0.168	1.212	0.998	0.000	9.931	197.347
2741	0.174	52.018	51.898	51.958	0.060	0.173	1.212	0.998	0.000	9.931	197.398
2921	0.178	52.148	51.833	51.991	0.158	0.177	1.212	0.994	0.000	9.931	197.335
3101	0.182	52.360	51.768	52.064	0.296	0.180	1.212	0.989	0.000	9.931	197.326
3281	0.185	52.557	51.753	52.155	0.402	0.184	1.212	0.985	0.000	9.931	197.360
3461	0.190	52.865	51.889	52.377	0.488	0.188	1.212	0.982	0.000	9.931	197.309
3641	0.194	53.331	52.177	52.754	0.577	0.192	1.212	0.978	0.000	9.931	197.402

Notes: Complete data available in the CONSOL folder in "Supplementary material." See Table T4 for heading definitions.



Table T6. Example data from shear phase of triaxial experiment TX729. (See table notes.)

Time (s)	$\varepsilon_a$ (%)	$q/\sigma_{vc}'$	$p'/\sigma_{vc}'$	$\Delta u_e/\sigma_{vc}'$	$\Delta u_s/\sigma_{vc}'$	$\Delta q/q_{max}$	$E/\sigma_{vc}'$	A	$\phi$ (°)	Area (cm <sup>2</sup> )
0	0.000	0.200	0.796	0.000	0.000	0.000	0.000	0.000	14.564	9.938
26	0.001	0.201	0.796	0.001	0.000	0.014	392.329	0.506	14.641	9.938
28	0.001	0.201	0.796	0.001	0.000	0.018	328.457	0.524	14.661	9.938
30	0.001	0.202	0.796	0.001	0.001	0.021	346.194	0.523	14.678	9.938
32	0.001	0.202	0.796	0.002	0.001	0.024	361.002	0.522	14.694	9.938
34	0.001	0.202	0.796	0.002	0.001	0.026	398.847	0.522	14.708	9.938
36	0.001	0.202	0.796	0.002	0.001	0.029	305.878	0.536	14.726	9.938
38	0.002	0.203	0.796	0.002	0.001	0.034	293.969	0.534	14.748	9.938
40	0.002	0.203	0.796	0.003	0.001	0.037	292.411	0.536	14.767	9.938
42	0.002	0.203	0.796	0.003	0.001	0.040	288.996	0.538	14.784	9.938
44	0.003	0.203	0.796	0.003	0.001	0.042	227.688	0.544	14.798	9.938
46	0.002	0.203	0.796	0.003	0.001	0.046	272.000	0.545	14.817	9.938
48	0.002	0.204	0.796	0.004	0.002	0.049	342.499	0.551	14.837	9.938
50	0.003	0.204	0.796	0.004	0.002	0.053	302.502	0.552	14.856	9.938
52	0.003	0.204	0.796	0.004	0.002	0.056	289.963	0.555	14.873	9.938
54	0.003	0.204	0.795	0.005	0.002	0.058	295.203	0.558	14.889	9.938
56	0.003	0.205	0.795	0.005	0.002	0.062	285.947	0.556	14.907	9.938
58	0.005	0.205	0.795	0.005	0.002	0.065	209.370	0.563	14.928	9.938
60	0.004	0.205	0.795	0.005	0.002	0.069	260.126	0.563	14.947	9.938
62	0.004	0.205	0.795	0.006	0.002	0.072	260.886	0.565	14.966	9.938
64	0.004	0.206	0.795	0.006	0.003	0.074	275.935	0.569	14.981	9.938
66	0.004	0.206	0.795	0.006	0.003	0.078	254.306	0.570	15.000	9.938

Notes: Vertical consolidation stress = 610 kPa. Complete data available in the SHEAR folder in "Supplementary material." See Table T4 for heading definitions.